Application of Queuing Theory of a Toll Plaza-A-Case Study

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Abstract— Due to ever increasing traffic, the road capacity has to be increased to accommodate different configuration vehicular dimensions. Toll roads need huge financing to construct a safe, effective, durable road network. Toll financing is one of the technique in which revenue collected from the road users for the service provided by them. This in turn results in development in queues at particular junction where in the toll booths are erected. Long queue could lead to increase in travel time which is drawback of road user. Hence toll should be designed and planned in such a way that minimum time would be wasted in the queuing area. The toll booths are planned on the basis of queuing area. Queuing theory involves parameters such as arrival, number of lanes, service time, waiting time, merging area. In present study road inventory, traffic volume, space mean speed, arrival rate, time headway and service rate are analyzed.

Keywords—Arrival rate, Service rate, Space mean speed, Time headway

CHAPTER 1 INTRODUCTION

1.1 General

A queue is simply a waiting line. Therefore systems that involve waiting lines are called queuing systems and mathematical descriptions of queuing systems are known as queuing models. Transportation systems often involve queues. Queuing or waiting-line, phenomena are everyday occurrences Queuing systems are characterized by an arrival pattern, a service facility and a queue discipline. Toll financing has been used throughout the history of civilization to make the building of long-distance roads possible. Beginning in the 1940's, America's first modern freeways were financed with tolls. Today developing nations such as China are building their own networks of superhighways, and they too turning to the tollbooth for expenditure. Tolls are being used successfully in places such as Singapore and London not just to finance road construction, but to limit the flow of vehicles into the urban core, increasing transit usage and unclogging the crowded streets [7].

Despite its many advantages, there is also disadvantage associated with tolling. When traffic is thick, vehicles backup in line to get to tollbooths, and after paying their tolls, drivers lose time scrambling for position as the many lanes exiting the toll plaza merge together, returning the road to its original width. A study conducted at the New Jersey Institute of Technology estimates that a travel time savings of 2 minutes, or over 10 percent, could be affected by the removal of two toll plazas along 14- mile section of the Garden State Parkway [10]. Modern toll facilities, such as Highway 407 near Toronto and the SR-91 Express lanes in Orange County, California, require all payment to be made by means of electronic transponder, so that vehicles do not have to slow down in order to pay the toll [7]. But on many older toll ways moving to all-electronic payment is not an option, while mounting congestion means that planners are faced with the problem of configuring their existence infrastructure to provide the best possible service.

1.2 Background

Highway toll plazas constitute a unique type of transportation system that requires special analysis when trying to understand their operation and their interaction with other roadway components. On the one hand, these facilities are one of the most effective means of collecting user fees for roadways. The object of a toll highway should be to minimize average travel time of all drivers on that road. On the other hand, toll plazas adversely affect the throughput or capacity of the facilities they serve. The adverse effect of toll plazas is particularly evident during hours when traffic is usually heavy. Thus highway toll experience lengthy vehicular queues and long delays when demand is near or exceeds processing capacity. Efficient sizing of toll plazas becomes critical in minimizing the space requirements and capital expense of collecting user fees. Hence keeping all these in view an effort has been made to study the performance of an existing toll on National highway-75 near Kadaballi between Bangalore to Mangalore stretch by applying the queuing theory.

1.3 Scope of Study

Toll plazas have become means for collecting revenue in order to build network of roadways which in turn improve safety, comfort, reduce average travel time and improve the capacity of roads. The study of queuing is important to find out new design methods in arranging the toll plazas and means of operating the toll plazas which in turn improve the service rate at the toll booth and the capacity of the tollbooth otherwise would have created long queues. In the present study an attempt has been made in understanding how the toll booth works which is being designed on queuing theory. The study involves collecting the geometric attributes of the toll plaza, the arrival pattern of vehicles to the queuing area, the service provided by the system.

1.4 Study Area

The site selected for the project lies near Kadaballi between Bangalore to Mangalore highway. The stretch of road length is considered to be 500m away from the toll plaza. This provides clear view of the place selected for case study of

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queuing theory. Ten service booths fixed by the company called L&T Devihalli-Hassan pvt.ltd. There are some irrigated land farms agricultural lands around the toll gates. The selected road is a divided four lane National Highway (NH-75). This site has been selected for the study purpose of queuing theory because there are no intersections near toll plaza. The length of each toll booth is 4.2m and width 1.9m. The length of each lane is 3.6m.



Figure 1.1: Toll plaza of the Study Area.

1.5 Methodology

The major steps involved in the present study are

- Road inventory of the selected road section
- Traffic volume count as per IRC:9-1972 "Traffic Census on Non-Urban Roads"
- > To find out the velocity of approaching vehicles by Space mean speed.
- To find out the inter-arrival rate of vehicles by taking time headway.
- To find out the Service rate provided by the serving system at the toll booth.
- > To analyze the performance of toll booth from the collected data.

LITERATURE REVIEW

CHAPTER 2

2.1 General

A primary objective in operational problems involving flow is to ensure that the average capacity can handle the average flow, so that persistent traffic jams do not occur. Queuing theory was developed in order to describe the behaviour of a system providing services for randomly arising demands. The fundamental idea of the theory is that delay in a system is caused by an interruption in the flow pattern. Queuing theory is almost exclusively used to describe the traffic behaviour at signalized and un-signalized intersections [10].

2.2 Characteristics Of Queuing System

The analysis of queuing systems and its variables has been the focus of many studies and researchers for many decades. The solution to a queuing problem entails the assessment of a system's performance, which in turn is described by a set of measures of performance (MOP) [2]. The inputs include

- 1. The input function (Arrival rate)
- 2. The input source (Finite/Infinite)
- 3. The queue discipline (FIFO/LIFO)
- 4. The channel configuration (Number and Arrangement)
- 5. The delay time (Service rate)

The basic component or main parameters of a queuing system is shown by the figure 2.1

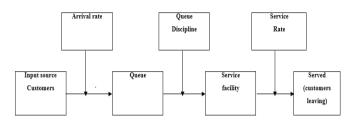


Figure 2.1: Components of a basic queuing system.

Mean Arrival Rate: It is rate at which customers arrive at a service facility. It is expressed in flow (Vehicles/hour) or time headway (Seconds/vehicles). If inter arrival time that is time headway (h) is known the arrival rate can be found out from the equation.

$$\lambda = \frac{3600}{h}$$

- Queue Discipline: queue discipline is a parameter that explains how the customers arrive at a service facility. The various types of queue disciplines are
- 1. First in first out [FIFO]
- 2. First in last out [FILO]
- 3. Serviced in Random order [SIRO]
- 4. Priority Scheduling

First in first out: If the customers are served in the order of their arrival, then this is known as the first-come, first served (FCFS) service discipline.

First in last out: Sometimes, the customers are serviced in the reverse order of their entry so that the ones who join the last are served first.

Served in Random order: Under this rule customers are selected for service at random irrespective of their arrivals in the service system. In this every customer in the queue is equally likely to be selected. The time of arrival of the customers is, therefore of no relevance in such a case.

Priority service: Under this rule customers are grouped in priority classes on the basis of some attributes such as service time or urgency or according to some identifiable characteristics and FIFO rule is used within each class to provide service.

- > Numbers of servers: The number of servers that are being utilized should be specified and in the manner they work that is they work as "Parallel" servers or "Series" servers has to be specified.
- Mean service rate: It is the rate at which customers depart from a transportation facility. It is expressed in flow (vehicles/hour) or time headway (seconds/vehicle). If inter-service time that is time headway (h) is known, the service rate can be found out from the equation.

$\mu = \frac{3600}{h}$

2.3 Structuring Of Queuing Model

Traditionally, traffic flows are modelled empirically, using origin-destination matrices [9]. One of the most important equations in traffic flow theory is that relating between traffic flow (q), traffic density (k) and traffic speed (s) which is given as

$q = k \times s$

These fundamental parameters of a traffic flow can be used as inputs in developing appropriate queuing models. Queuing models are often referred to using the Kendall notation, consisting of several symbols e.g. M/G/1 [9]. The first symbol describes the arrival rate of traffic into a system, the second for the service rate provided by the system to the vehicles while the third indicates the number of servers in the system.

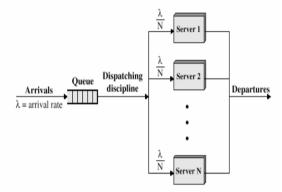


Figure 2.2: A simple model of a multi-server queuing system.

2.4 Theory Applied In Queuing Model

Poisson distribution

- The experiment results in outcomes that can be classified as successes or failures.
- The average number of successes that occurs in a specified reason is known.
- The probability that a success will occur is propositional to the size of the region.
- The probability that a success will occur in an extremely small region is virtually zero.

2.5 The Toll plaza

Tolls are systems (or sometimes called as barriers) constructed on roads which are meant to provide facilities to road users by reducing their average travel time, increased speed, safety and improve the capacity of the road sections. Tolls have become a means of generating revenue in building expressways and National highways there by reducing the problem of congestion on many of the existing road networks.

There are two types of toll collection systems available. These are

- 1. Open toll system
- 2. Closed toll system

Open toll system: In an open toll system, not all patrons are charged a toll. In such a system, the toll plaza is generally located at the edge of the urban area.



Figure 2.3: Showing the Open Toll System.

Closed toll system: In a closed toll system, patrons pay the toll based on miles of travel on the facility and category of vehicle. In a closed toll system, plazas are located at all the entry and exit points, with the patron receiving a ticket upon the entering the system. Upon exiting, patron surrenders the ticket to the collector and is charged a prescribed fee based on category of vehicle and distance travelled [3].



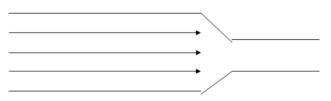
Figure 2.4: Showing the Closed Toll System.

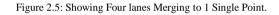
2.5.1 Approaching the Toll Plaza

The highway is generally assumed to be free flowing on either side of toll plaza. This assumption allows congestion resulting from the toll plaza design to be isolated from general congestion on the highway. Generally the toll plaza is designed in such a way that traffic flow levels through them will be less than the capacity of the highway. Since most of the toll plazas particularly in India are cash –collecting, at some point most vehicles must stop either because the vehicle in front of them has stopped or they have reached a toll booth. This in turn leads to building up of queue lengths particularly when the instantaneous demand exceeds the service. So it is necessary to find out the queuing area which is essential in fixing the number of servers in a toll plaza.

Merging: After the toll booths, the roadway must narrow back from a number of lanes equal to the number of tollbooths, to its normal width, a section will called as "merging area". Sometimes the extra lanes end almost immediately, forcing a sharp merge at a relatively low speed. There are three different merging patterns are used when lanes begin and end are,

- With several lanes merging into one, all of the merging could occur at a single point, but this means that as many vehicles as there are lanes could interfere with each other at that point.
- One common choice is to always merge out the rightmost (or leftmost) lane until the desired number of lanes is reached.
- Another possibility is a "balanced" pattern where pairs of adjacent lanes all across the roadway merge repeatedly until the desired roadway width has been attained.





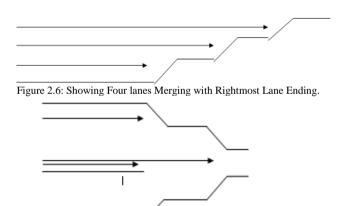


Figure 2.7: Showing Four lanes merging in a Balanced Pattern.

Gustavo ceballos et.al, in "Queue Analysis at Toll and Parking Exit plazas: A Comparison between Multi-server Queuing Models and Traffic simulation." have shown that the simple analytical models can be used for initial understanding of the queuing system but warn that good judgement must be employed while using the analytical models, as their results may differ significantly from real-life plaza operation. They insist the analytical models to be avoided when trying to analyse toll plaza operation under high levels of demand (0.90 < v/c ratios < 1.0) given their asymptotic behaviour within this range. The traffic simulation provides a more comprehensive understanding of the toll plaza operation allowing for a more in-depth analysis of its performance. According to them simulation should be used for advance planning, design, operation and management of toll and exit plazas facilities.

Abdul aziz, A.R., et.al, in "Application of queuing theory to vehicular traffic at signalized intersection in Kumasi-Ashanti region, Ghana" has shown that queuing theory can be applied in modelling the vehicular traffic flow and minimize vehicular traffic in order to reduce delays on roads of Kumasi-Ashanti region. The analysis of the data collected at Oforikrom intersections revealed that a smooth flow of traffic is seen when the server at each channel is able to serve more than cars in waiting queue. But in evening there is restriction to flow due to the restraints caused by the commercial vehicle drivers. They have suggested that use of public transport by the government of Ghana would help in reducing congestion on the roads, which in turn boost the productivity.

NicoVandaele.,et.al, in "A Queuing Based Traffic Flow Model" have shown that queuing models can be applied in assessing the traffic flow parameters compared to traditional empirical methods, which lack in terms of predictive power and the possibility of sensitivity analysis. Based on queuing theory they analytically constructed the well-known speedflow-density diagrams. They have shown that the exact shape of the different speed-flow-density diagrams is largely determined by the model parameters so that a good choice of parameters can help to adequately describe reality. They also believe that speeds have a significant influence on vehicle emissions and models can be effectively used to assess the environmental impact of road traffic.

CHAPTER 3 FIELD STUDIES

3.1 General

In order to understand the theory behind queuing a toll plaza has been selected in the present study which is located on NH-75 near Kadaballi. Field studies like road inventory, traffic volume, space mean speed, time headway, arrival rate and service pattern have been carried out.

3.2 Road Inventory

Road inventory reflects the pavement characteristic. Inventory data basically consists of data necessary to identify the project under evaluation. This consists of the geometric details of the project which are collect visually walking along the entire stretch. All of these data will remain constant until the pavement undergoes maintenance or repair. The inventory data of the selected road stretches are listed in table-3.1

Roadway attributes covering the roadway classification, ownership, physical condition, traffic volume, pavement conditions, highway performance monitoring information and more.



Figure 3.1: Showing the Flexible Pavement and rigid pavement near the Toll Plaza.

Table 3.1: Shows the details of Pavement Structure before
Toll Plaza.

SI no	Parameters	Collected data
1	Type of pavement	Flexible pavement
2	Divided/undivided	Divided
3	Number of lanes	Four
4	Width of pavement (m)	9
5	Median width (m)	2.5
6	Shoulder width (m)	1.5
7	Type of shoulder	Earthen

Table 3.2: Shows the de	etails of Queuing Area.
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S1	Parameters	Collected data
No		
1	Type of pavement	Rigid
2	Width of pavement(one side)	30m
3	Length of pavement on arrival side	250m
4	Length of pavement on merging side	250m
5	Number of toll booth on one side	5
6	Length of each toll booth	2.4m
7	Width of each toll booth	1.9m
8	Width between toll booths	3.6m
9	Type of merging	Left most merging

Table 3.3: Showing traffic flow on both the directions of the National Highway 75.



Figure 3.2: Shows the Toll Area having Concrete Road.

3.3 Traffic Volume:

Traffic volume or traffic flow is defined as "the product of the average traffic intensity and the time period of the study". It is measured by the units "vehicle per hour". In the present study the traffic count census is done as per IRC: 9-1972 "Traffic census on Non-Urban Roads". To take into account the randomness, the traffic volume study was carried out in short intervals (1 hour) at different hours of a day and at different days. Traffic flow is usually considered to be roughly constant at any given instant, as changes in flow occurs smoothly and slowly, while measurements employed are over very short time periods.



Figure 3.3: Shows Vehicle Platoon at Toll Plaza.

Table 3.3: Showing traffic flow on both the directions of the
National Highway 75.

SL	Direction	Time	Traffic flow	Average traffic flow			
No			rate(vehicles/hour)	rate (vehicles/hour)			
1		4:00 to 5:00	396				
	Hassan to	5:00 to 6:00	518	275			
	Bangalore	9:00 to 10:00	375				
		10:00 to 11:00	262				
2		4:00 to 5:00	297				
	Bangalore	5:00 to 6:00	311	345			
	to Hassan	9:00 to 10:00					
		10:00 to 11:00	417				

3.4 Space Mean Speed:

The space-mean speed is "the average speed of vehicles travelling a given segment of roadway during a specified period of time". It is calculated using the average travel time and length for the roadway Segment. In this study the space mean speed is found out as per IRC: 108-1996"Guidelines for Traffic Prediction on Rural Highways". The space-mean speed is then calculated by dividing the distance between Instrumented locations by the average travel time. This is speed that is involved in flow-density relationships. The data of the space mean speed collected in the field is shown from table 3.5

Space mean speed= <u>
Distance in m</u> <u>
Average time taken in seconds</u>

 $V_s = \frac{3.6 \cdot d \cdot n}{\sum_{i=1}^{N} T_i}$

Where VS=Space mean speed km/s $T_{i=t_1+t_2+t_3+t_4+...+t_n}$ =Average

time taken seconds.



Figure 3.4: Collection of Space-mean Speed at the Study Area.

Table 3.4: Showing the Space-mean Speed of Vehicles
approaching the Toll Booth Near to Toll.

Duration (hours)	Direction	Average time (s)	Distance in (m)	Number of Vehicles	Average velocity near the toll booth (km/hr)
4	Hassan to	0.80	10	453	20.38
	Bangalore				
4	Bangalore to	0.96	10	366	13.72
	Hassan				

Table 3.5: Showing the Space-mean Speed of Vehicles approaching the Toll Booth Away from Toll.

	out ing in		· · · · ·		
Duration (hours)	Direction	Average time	Distance in	Number of	Average velocity
		(s)	(m)	Vehicles	away
					from
					toll
					booth
					(km/hr)
4	Hassan to	0.63	10	598	34.17
	Bangalore				
4	Bangalore	0.76	10	556	26.33
	to				
	Hassan				



Figure 3.5: Collection of Space-mean Speed data by marking the distance.

Table 3.6: Showing the Details of the Space Mean Speed (Hassan to Bangalore).

Type of vehicle						Four wh	ieelers					Buses		Trucks		
Parameters																
Total vehicles						10	1					1	9	22		
Distance						1	0		10							
Time in seconds	.99	.92	.58	.77	.57	.78	.57	.58	.56	.51	.66	.83	1	1.2	1.7	1.2
	.53	.58	.64	.67	.62	.59	.71	.61	.6	.63	.72	1.4	.95	1.78	.92	15
	.78	1.1	.57	.66	.58	.83	.60	.72	.84	.52	.74	.87	.97	.8	.57	1.7
	.56	.66	.69	.54	.45	.52	.68	.53	.12	.57		1.09	.96	1.02	1.6 5	15
	.53	.68	.59	.57	.66	.55	.70	.75	.56	.63		.99	.83	1.46	1.8 0	
	.78	.73	.63	.54	.54	.55	.75	.62	.55	.59		1.0	.81	1.25	1.7	
	.89	.61	.56	.65	.64	.74	.75	.56	.64	.61		1.09	.970	1	13	
	.59	.95	.78	.63	.85	.61	.80	.54	.52	.72		.93	1	.80	.87	
	.58	.57	.57	.60	.56	.67	.61	.64	.56	1.6		1.9		1.41	1.1 9	
Average time taken						.7)					t		.73		
Space mean speed						14.3	28					1	0	13.64		

Table 3.7: Showing the Details of the Space Mean Speed (Hassan to Bangalore).

Type of vehicle			Foi	ar wh	eelers					Buse	6	Trucks							
Parameters																			
Total vehicles		45								13		38							
Distance				10		_				10		10							
Time in seconds	.3	.7	.5	.6	.6	.7	.3	.5	.4	.4	.б	.9	1.2	1.8	1.2	2	1.2	.8	
	.3	.2	.3	.2	.6	.7	.2	.3	.5	.6	-	1.3	1.1	.9	9	.6	1.5	.7	
	.2	.4	.5	.3	.6	.4	.5	.5	.4	.6	-	.9	.7	.5	.9	.7	.7		
	.3	.4	.6	.3	.3	.6	.6	.4	.7	1		1	.8	.4	.7	.5	.6		
	.4	.6	.3	.7	.4	.4	.4		.4	.5		.6	.7	.8	9	.6	.7		
	.3	.4	.3	.3	5.3	.5	.3		.4	.5		1	.7	2.5	.7	.8	1.2		
Average time taken		.45								.615		.939							
Space mean speed		22.05								16.25 10.64									

Table 3.8: Showing the Details of the Space Mean Speed (Hassan to Bangalore).

Type of vehicle Parameters		Four wheelers									Trucks													
Total vehicles				41				18 40																
Distance		10							10		10													
Time in seconds	.79	2.2	.96	.98	.72	.96	.68	.84	1	1.5	1.3	1.7	1.6	1.7	.78	.96	1.4	1						
	.69	.74	.82	.75	.69	.68	.7	.89	1	1	.98	1.4	.92	.76	.86	1.8	2.8	1.1						
	.79	.96	.6	.78	.58	.62	.72	.96	.88	.95	.97	.99	1.2	2.5	1	1.4	1	-						
	1	2.2	1.9	.65	.59	.76	.54	.95	.95	1.1	1.8	.86	1.0	.97	.74	2.8	.92							
	.76	.57	.61	.69	.80	.72	.68	.85	.96		1.5	1	1.2	1.7	4	1	1.4	-						
	.74	.8	.74	.74	.74	.97		.85	.95		1.1	1.6	1.4	3	1.2	.92	2.3	-						
Average time taken		.84							.98		1.41													
Space mean speed		11.90													7.09			7.09						

Table 3.9: Showing the Details of the Space Mean Speed (Bangalore to Hassan).

Type of vehicle Parameters				F	our w	heeler	'8					Bu	ises				Truck	cs	
Total vehicles					5	8						2	1				30		
Distance					1	0						1	.0				10		
Time in seconds	.61	.67	.61	.74	.84	.64	.51	.66	.56	.43	1	1	.91	.92	.87	.96	1.3	1.7	1.7
	.60	.66	.8	.71	.6	.68	.55	.8	.9	.45	1.1	1	.94	.99	1.2	1.3	1.8	1.7	1.4
	.64	.79	.79	.66	.72	.59	.62	.59	.94	1	1	.99	1.1	.92	1.4	1	1.2	1.6	1.1
	.74	.63	.65	67	.68	.66	.71	.76	.75		.89	.97	.97		.99	1.4	1.5	1.7	1.6
	.62	.63	.66	1.6	.68	.6	.62	.85	.77		1	.88	1.2	-	1.6	1.3	2.3	1.2	1.2
	.62	.57	.83	.53	1	.55	.46	.73	.93		.83	.92	1		.73	1.4	1.7	1.7	1.1
Average time taken		.68											99				13	8	
Space mean speed					14	17						1	0.10				7.2	4	

Table 3.10: Showing the Details of the Space Mean Speed (Bangalore to Hassan).

Type of vehicle		F	our wh	eelers		Bus	es		Trucks	
Parameters										
Total vehicles			30			7			18	
Distance			10			10)		10	
Time in seconds	.75	.56	.74	.91	.8	1.2	1.05	.99	1.5	1.2
	.72	1	.62	.63	.77	.95		1.62	1.4	1.8
	.58	1	1	2	.72	1.3		1.6	1.3	13
	1.38	.56	.62	.77	.64	1.9		1.2	1	1.2
	.60	1	.69	.68	.84	.82		.91	1	1.6
	.33	.76	.78	.69	.78	1.1		1	1.6	1.3
Average time taken			.84			1.1	8		1.322	
Space mean speed			11.8	7		8.4	2		7.55	

Table 3.11: Showing the Details of the Space Mean Speed (Bangalore to Hassan).

Type of vehicle Parameters]	Four wh	eelers				Buses			Tru	ks	
Total vehicles				42					17			20)	
Distance				10					10			1()	
Time in seconds	.93	.81	.61	.6	.7	.88	.69	1.8	.88	.83	1.2	.89	1	.84
	.87	.5	.64	.5	.78	.48	.65	1	.96	1.1	15	1.4	1.2	
	.5	.52	.48	.87	.69	.66	.81	1.2	.86	.9	1.9	1.7	1.5	
	.67	.56	.64	.62	.53	.7	.76	1	.69	.85	1.7	1.7	.88	-
	.57	.6	.71	.86	.76	.59	.70	.93	.77		1.4	1	1.8	
	.8	.51	.82	.47	.51	.62	.52	1	.86		1	1.4	1	
Average time taken				.65					.98			1.3	3	
Space mean speed				15.3	5				10.12			7.4	6	

Table 3.12: Showing the Details of the Space Mean Speed (Bangalore to Hassan).

Type of vehicle Parameters				I	Four	whe	elers					J	Buses					Truck	15		
Total vehicles						66							17					41			
Distance						10							10					10			
Time in seconds	1.2	.6	.4	.4	.6	.4	.7	.4	.3	.5	.3	1	.6	.5	.8	1.2	1	.5	1	.5	.9
	0.6	.7	.7	.4	.6	.3	.3	.4	.3	.4	.5	.8	.5	.1	9	.8	1	.4	1.5	.5	1.2
	.7	.4	.3	.8	.3	.3	.4	.4	.7	.3		.7	.1	.1	.6	.9	1.2	.7	1.2	1.1	.5
	.7	.4	.5	.4	.3	.8	.7	.5	.5	1		.7	.6	.5	.6	1.7	1.2	.7	1.2	1	.5
	.3	.4	.4	.5	.4	.3	.5	.4	.3	.4		.5	.8	1	.5	.9	1.4	.6	1.2	1.3	1.1
	.6	.3	.3	.3	.5	.5	.3	.2	.5	.2		.8	.7		1	.7	.6	1.9	.7	1.3	
Average time taken		.56									.7					.9	49				
Space mean speed		17.74											14.2	8				10	.53		

Table 3.13: Showing the Details of the Space Mean Speed
(Hassan to Bangalore).

Type of vehicle Parameters				Fou	r wh	eeler	5					Buses					Tı	rucks		
Total vehicles					54							30						32		
Distance					10							10						10		
Time in seconds	.5	3	.6	.3	.3	.4	.6	.3	.3	.5	.5	1.1	1	.6	.8	.9	1.4	1.6	1.2	1.2
	.4	.5	.3	.5	.6	.3	.4	.4	.3	.5	.8	.5	.5	.6	.6	.5	.6	.6	.6	.5
	.6	.4	.1	.3	.5	.5	.3	.7	.3	.7	.6	1.2	5		1.2	1.2	1.4	.7	1.2	-
	.3	.3	.4	.3	.1	.3	.7	.3		.7	.5	.7	.5		.7	1.3	.7	.4	.7	
	.3	3	.3	.4	.3	.3	.4	.6		.5	.1	.5	1		1	.9	1	.8	.6	
	.1	3	.3	.3	.3	.3	.4	.3		.7	.5	.5	1		.8	1	1.2	1	.6	
Average time taken		.4										.677					_	934	-	
Space mean speed		25										14.77					1	0.7		

3.5 Time Headway:

Time headway (H) is the difference between the time the front of a vehicle arrives at a point on the highway and the time the front of the next vehicle arrives at the same point. The time headway is usually expressed in seconds. Time headway is necessary to known the inter arrival rate among the vehicles which is needed to find out the capacity of a highway system.



Figure 3.6: Showing the Time Headway Count.

Table 3.14: showing th	ne Average T	ime Headway
1 abic 5.14. showing u	ic Average 1.	mic meauway.

			0		
Dura	From	То	Total Number	Total time	Average
tion(of vehicle	in seconds	time in
hour				(s)	seconds
s)					(s)
14	Hassan	Bangal	2068	21170	10.23
		ore			
	D 1		2270	21150	10.10
14	Bangalo	Hassan	2370	21179	10.42
	re				



Figure 3.7: Observing the Time Headway Count.

Table 3.15: Showing details of Time Headway.

Direction			H	assan	to Ba	ngalore						В	anga	lore t	o Hassan			
Lane		La	ne A				Lane	В			Lan	еC				Lane	D	
Parameters	Total vehicle s	Ti	me in	secor	ıds	Total vehicle s		Fime econ		Total vehicles	Tir	ne in	seco	nds	Total vehicle s		Time i second	
		4	22	5	10		7	25	24		1	37	89	60		31	22	50
		8	8	5	9		4	34	21		22	13	20	30		21	24	58
		6	1	45	33		11	31	4		34	15	30	34		12	18	
		10	4	13	9		13	21	18		17	56	12	17		71	19	
		8	6	16	45		7	10	32		32	6	16	4		11	35	
		9	7	37	9		16	33	24		14	27	6	38		21	9	
		14	5	4	57		35	11	2		22	47	4	30		19	11	
		4	3	47	2		9	3	21		34	14	57	5		21	7	
		1	16	13	10		15	17	14		13	20	16	4		20	79	
		24	3	34	44		53	12			2	17	16	7		77	31	
	164	9	36	20	8	102	14	21		126	7	30	7	11	88	11	3	
	104	15	24	36	6		73	6			18	11	46	6		10	48	
		12	23	38	10		45	3			14	98	14	65		9	27	
		30	34	6	8		30	17			38	48	19	10		11	73	
		5	14	9	8		20	57			8	26	17	75		12	35	
		61	37	31	18		53	18			5	16	39	66		52	51	
		2	3	10	3		52	5			27	13	16	28		26	17	

Table 3.16: Showing a details of Time Headway.

Direction			Has	san to	Bangalore							Bar	ngalor	re to F	Hassan		
Lane		Lane	A			Lane H	}			Ι	lane (2				Lane D	
Parameters	Total	Time	e in sec	onds	Total	Time	in se	conds	Total		Time	in sec	onds		Total	Time is	1 seconds
	vehicles				vehicles				vehicles						vehicles		
		61	20	4		17	41	28		44	16	20	11	16		36	15
		62	17	31		25	9			15	32	16	12	21		22	14
		8	5	62		7	10			35	31	12	41	55		68	20
		4	9	12		27	99			19	99	20	31	21		29	19
		4	28	12		8	6			41	78	35	9	6		53	39
		37	43	43		13	48			21	9	21	11	21		98	42
		20	6	14		57	9			29	39	11	99	19		7	21
		10	10	27		19	12			3	16	14	11	31		16	59
		15	24	13		23	35			89	17	10	9	29		57	99
		5	36	16		24	14			13	17	24	19	11		99	18
	108	36	19	15	86	13	10		206	12	19	51	30	8	68	49	59
		4	16	51		86	7			18	20	20	7	6		64	46
		45	13			79	3			4	6	25	24	19		16	31
		23	34			18	62			7	48	37	60	13		7	
		65	19			2	39			9	27	32	7	20		13	
		21	24			22	17			16	9	18	9	3		33	
		29	16			39	13			10	9	2	8	37		71	
		99	8			40	26			3	28	19	19	9		47	
		8	60			52	17			56	29	18	48	28		3	
		5	43			43	51			52	25	13	19	20		43	
		12	11			31	95			45	21	41	38			96	
		12				1	1			1	²¹	1	50			20	

Table 3.17: Showing a details of Time Headway.

Direction		_	H	assan	to Ba	ingalore		_				_	Ban	igalor	e to I	Hassan			
Lane		Lar	ne A]	Lane	В			L	ane C					Lane	D	
Parameters	Total vehicles	Ti	me in	secor	ıds	Total vehicles		Time : second		Total vehicles		Time	in sec	onds		Total vehicles	Tim	e in se	conds
		40	12	16	4		31	85	64		31	15	56	16	23		1	5	3
		17	9	3	11		38	99	49		18	43	5	76	38		2	3	78
		11	34	26	25		99	12	24		99	21	4	77	12		4	22	15
		19	44	31	11		66	99	65		16	6	19	10	38		6	2	26
		3	5	73	24		5	98	24		25	13	36	64	20		7	4	23
		12	26	54	53		13	3			43	10	40	8			1	2	46
		10	16	3	6		30	7			13	50	23	6			12	60	3
		12	25	35	85		32	10			18	25	37	16			14	6	30
		5	4	9	18		99	1			38	8	20	9			4	94	1
		18	31	22	6		6	53			23	28	60	12			7	10	60
	156	7	3	12	4	94	32	99		178	61	10	16	20		126	20	18	14
		19	5	19	8		99	85			3	35	12	47			4	5	5
		5	9	9	44		40	92			14	16	14	54			6	9	6
		18	18	32	20		9	99			34	22	17	8			10	18	99
		2	13	19	23		11	98			4	14	13	17			19	8	9
		12	29	11			65	11			13	4	43	38			15	94	12
		3	5	15			21	21			5	9	50	51			31	41	19
		16 76	16 36	7 15			99	36 99			14	24	12	46			25 2	14	38 99
							9 17	99			195	*	30	8 35			-	11	
		11 33	10 25	2 10			31	33			/ 16	26	14 8	50 31			1	28	4 22
		33	23	10			51	33			10	21	δ	51			1	1 21	22

Table 3.18: Showing a details of Time Headway.

Direction										Hase	san to	Bangalore										
Lane					Lar	e A										Lan	B					
Parameters	Total vehicles				Ti	me in	secor	ıds				Total vehicles				Ti	ne in	seco:	nds			
		11	56	27	9	30	6	32	5	2	14		10	8	11	19	10	12	13	1	4	43
		26	18	18	27	20	7	13	16	21	11		13	8	6	13	19	3	35	5	4	9
		31	8	1	40	21	5	19	16	5	89		10	17	11	38	2	30	21	7	1	40
		14	9	4	27	18	12	59	65	4	56		5	21	7	4	13	11	23	19	16	27
		8	17	22	3	30	1	7	54	14	2		65	26	5	11	36	5	21	16	26	3
		97	23	31	14	4	15	4	67	10	43		2	57	72	2	14	11	9	12	13	
		3	18	26	17	4	5	2	7	26	23		51	8	3	14	2	1	10	12	15	
		99	91	15	7	5	4	45	10	62	98		60	4	5	1	35	13	19	3	7	
		18	24	17	33	18	4	41	9	10	16		26	64	7	27	7	8	2	30	10	
	414	20	35	2	11	20	1	6	33	34		358	61	6	13	4	2	4	13	11	8	
		30	15	31	1	12	16	7	14	11			56	3	20	16	5	11	36	5	17	
		9	60	28	2	18	16	31	28	5			13	17	6	11	11	2	14	11	13	
		40	33	25	33	11	18	4	30	13			7	10	6	2	2	14	2	1	18	
		20	91	14	10	9	31	28	7	13			37	54	12	26	23	1	35	19	4	
		13	25	30	25	35	38	39	60	15			6	37	16	5	4	27	6	16	7	
		37	20	22	34	30	15	21	22	19			5	10	2	7	30	4	2	12	12	
		19	34	13	13	15	19	22	8	10			4	61	11	13	1	16	5	24	19	
		34	99	4	11	6	28	5	10	22			61	5	17	35	5	11	11	7	30	
		20	16	7	1	26	10	18	18	18			10	3	38	21	7	2	2	28	13	
		11	17	13	34	2	12	9	33	23			35	13	10	21	19	26	23	14	22	
		12	11	43	33	19	10	18	5	15			97	29	24	23	16	4	4	21	4	
		11	13	19	29	2	19	6	49	49			22	3	10	6	12	8	30	24	7	

Table 3.19: Showing a details of Time Headway

Direction									В	angal	ore to	Hassan									
Lane					Lar	ie C									Ι	Lane I)				
Parameters	Total				Ti	me in	secon	ıds				Total				Time	e in se	conds			
	vehicles											vehicles						_			
		14	12	14	13	9	6	2	5	2	19		9	22	8	27	8	7	40	32	9
		18	28	4	17	17	8	9	6	4	13		30	2	4	8	6	6	5	3	4
		12	8	6	6	7	8	18	7	8	19		19	6	2	9	7	15	12	6	21
		15	19	8	6	7	10	14	9	9	11		1	2	17	4	22	7	9	24	16
		5	30	8	7	16	15	9	13	44	10		11	3	2	6	34	21	30	1	4
		4	12	1	12	23	2	26	12	4	15		14	6	12	12	9	4	9	4	
		10	20	10	14	13	9	5	35	21	4		2	33	9	3	10	2	7	6	
		7	21	3	39	11	8	21	7	11	15	366	15	8	2	1	12	14	15	9	
	412	26	18	11	16	5	10	10	19	3			10	10	24	5	2	7	30	50	
		11	17	17	1	13	17	4	40	5			16	4	6	3	11	15	5	14	
		15	10	15	26	10	1	6	7	6			20	8	2	21	23	16	7	6	
		2	5	6	31	9	3	7	15	7			20	6	22	15	17	6	5	15	
		10	2	21	7	5	13	4	12	9			3	7	37	27	1	13	2	3	
		13	8	4	11	4	11	2	19	13			8	13	7	14	17	1	6	7	
		18	5	5	8	10	14	3	8	12			9	1	2	18	21	7	8	4	
		14	10	5	11	16	12	8	21	35			15	3	8	5	4	38	7	28	
		7	6	8	11	4	2	10	18	7			16	3	11	12	14	5	19	3	
		2	12	15	13	4	3	44	13	19			20	23	7	3	7	28	17	11	
		13	1	13	12	23	5	4	11	40			19	5	18	1	11	8	4	19	
		17	3	3	10	7	31	21	19	7			19	4	1	4	3	4	12	15	
		10	12	12	31	2	5	11	10	15			12	18	20	2	13	7	3	43	
		2	6	4	15	10	9	3	14	12			10	15	10	7	6	7	10	11	

Table 3.20: Showing a details of Time Headway

Direction							Ha	ssan to	Bangal	ore							
Lane			Lane A Lane B										e B				
Parameters	Total vehicles			Т	ime in	second	ls			Total vehicles							
		18	20	7	29	12	13	18	24		20	36	21	4	6	60	
		40	10	9	12	9	21	2	27		4	19	15	99	12	3	
		4	4	21	22	10	6	4	15		9	22	132	11	30	24	
		6	6	7	5	1	24	2	3		51	34	37	55	8	9	
		5	8	9	6	3	39	13	5		37	99	34	80	10		
		44	11	62	3	3	24	18	11		99	23	25	8	10		
		17	8	35	1	4	16	14	18		18	12	4	6	8		
		10	3	17	8	17	4	17	8		15	50	59	70	26		
	338	14	37	19	9	24	11	6	23	228	99	14	58	15	27		
		17	18	6	19	15	1	25	5		24	56	10	28	28		
		2	43	27	4	22	5	48	2		11	3	56	6	99		
		24	58	31	5	3	8	7	11		3	12	15	9	10		
		34	23	33	27	11	6	2	4		27	14	24	20	7		
		19	39	9	35	8	18	9	13		56	51	12	23	10		
		69	29	5	1	46	7	25	9		49	5	86	10	15		
		48	5	24	13	9	5	34			34	21	2	19	10		
		19	7	1	15	4	6	5			12	20	3	10	5		
		18	13	2	34	9	2	22			7	20	32	5	9		
		42	13	1	27	7	20	9			36	11	6	5	58		
		17	42	24	1	3	8	5			2	10	11	56	21		
		36	25	58	12	28	11	3			65	20	21	10	20		
		45	29	8	11	7	24	7			7	9		26	5		

Table 3.21: Showing a details of Time Headway.

Direction								I	Bangal	ore to	Hassan								
Lane				L	ane C									Lar	ie D				
Parameters	Total vehicles				Time	in se	conds				Total Time in seconds vehicles								
		13	24	6	4	3	7	11	4	4		10	154	15	35	11	17	16	16
		3	5	12	3	14	8	3	10	21		19	5	10	1	6	2	6	7
		9	4	15	7	20	9	7	3	5		18	12	4	3	8	4	18	32
		5	5	6	4	1	2	13	15	9		1	12	9	3	9	9	13	3
		19	5	4	24	13	3	7	15	7		25	5	1	11	6	12	22	34
		9	4	10	16	12	26	10	11	4		9	6	8	9	32	6	34	5
		19	12	10	3	3	5	1	22	15		7	7	23	3	5	3	21	53
		16	6	2	8	3	12	15	12	10		3	12	3	3	2	21	4	6
	370	19	10	17	10	7	17	9	24	29	336	1	7	20	16	1	29	47	1
		20	7	5	11	6	10	11	4	22		71	13	13	26	1	11	3	66
		19	3	1	24	6	25	13	10	7		5	15	5	20	11	262	16	42
		17	5	4	8	12	14	10	12	2		15	33	12	5	5	1	8	22
		7	9	6	5	7	30	20	22	14		51	46	13	3	22	6	17	6
		11	6	2	10	18	21	8	42	13		3	11	23	2	312	6	8	9
		4	13	10	12	39	36	8	7	7		30	1	9	12	2	29	31	16
		7	6	6	15	2	2	23	4	15		1	15	2	15	2	16	2	19
		12	10	3	19	17	19	8	14	2		28	1	18	4	8	6	16	6
		8	7	5	17	14	9	10	19			2	9	8	14	6	20	29	21
		26	4	8	6	8	2	24	2			5	7	9	20	2	11	4	11
		18	4	7	11	16	7	23	14			16	12	20	17	18	15	17	20
		5	6	20	6	22	7	3	17			18		5	10	11	5	8	31

3.6 Arrival Rate:

Arrival is generally defined as "The simple model assumes that the number of arrivals occurring within a given interval of time follows a Poisson distribution". This parameter is the average number of arrivals in time which is also the variance of the distribution.

Table 3.22: Showing the Average Arrival Rate.

Duration (hours)	From	То	Timings in seconds(s)	Observed vehicles
5	Hassan	Bangalore	18000	1973
5	Bangalore	Hassan	18000	1946



Figure 3.8: Showing Counting of Vehicles for every 30 seconds Time Interval

Table 3.23: showing the observed frequency from Hassan to Bangalore.

				D	anga	nore.			
Number of			Observe	d frequer	ıcy		Total number	probability	Theoretical
vehicles arriving for every	Day 1	Day 2	Day 3	Day 4	Day 5	Total	of vehicles		Frequency
30 second									
0	6	7	8	2	3	26	0	0.036	21.52
1	11	10	21	13	10	65	65	0.120	71.76
2	21	22	22	32	31	128	256	0.199	119.00
3	19	16	23	35	34	127	381	0.220	131.56
4	24	20	24	20	24	112	448	0.183	109.43
5	16	19	7	9	12	63	315	0.121	72.35
б	13	15	5	6	4	43	258	0.067	40.062
7	3	5	8	3	0	19	133	0.031	18.53
8	2	3	2	0	1	8	64	0.013	7.74
9	2	2	0	0	0	4	36	0.0048	2.87
10	0	1	0	0	0	1	10	0.0016	0.956
11	0	1	0	0	0	1	11	0.00048	0.287
12	1	0	0	0	0	1	12	0.000135	0.08
Above 12	0	0	0	0	0	0	0	0.00013	0.077
Total				598			1989	1.000	597

Table 3.24: showing the observed frequency from Bangalore to Hassan.

Number of			Observed	frequency			Total number	probability	Theoretical
vehicles arriving for every 30 second	Day 1	Day 2	Day 3	Day 4	Day 5	Total	of vehicles		frequency
0	0	0	12	12	12	36	0	0.02	11.58
1	1	2	14	18	15	50	50	0.078	45.16
2	7	1	25	40	30	109	218	0.153	88.58
3	12	11	21	22	23	89	267	0.199	115.22
4	15	16	20	16	20	87	348	0.195	112.90
5	21	20	10	4	15	70	350	0.152	88.008
6	16	18	5	6	3	48	288	0.099	57.32
1	12	15	2	1	1	31	217	0.055	31.84
8	13	12	0	0	1	26	208	0.027	15.63
9	6	11	0	0	0	17	153	0.011	6.36
10	6	5	0	1	0	12	120	0.0046	2.66
11	1	2	0	0	0	3	33	0.0016	0.926
12	0	1	0	0	0	1	12	0.00053	0.306
Above 12	0	0	0	0	0	0	0	0.00016	0.092
Total			5	79			2264	1.00	577

3.7 Service Rate:

The service rate depends upon the type of operation involved in providing service to the customers. Generally cash collecting service takes more time than the automatic way of collection. Service rate denotes the rate at which vehicles are been served in a system. It is the reciprocal of the service time.

When the vehicle enters the toll plazas, a rational driver selects the counter service by seeing the queue length existing relative to other counters. Once the vehicle is in the queue length it has to follow the queue discipline. The waiting time is the time spend by the vehicle in the queue length and the time spends in providing the amount. The driver must pay the with exact change in order to minimize service time.

Table 3.25:	Showing the	Average Service Rate.
1 4010 5.25.	billo willing the	riverage bervice reace.

Duration	From	То	Number	Total	Service time in
(hours)			of	time in	seconds(s)
			vehicles	seconds(
				s)	
4	Hassan	Bangalo	315	5660	18.02
		re			
4	Bangalor	Hassan	277	4709	17.06
	e				



Figure 3.9: Showing the Service Section of Toll Plaza.

Table 3.26: Showing the Details of Service Rating Time.

						Servic	e Ratin	g Time					
Type of Vehicles			Hassa	n to Bar	igalore				Ba	ingalore	e to Has	san	
	24.6	27.0	19.7	19.4	15.4	26.0	11.9	35.3	07.0	08.0	10.7	12.6	6.95
	22.0	08.7	12.1	09.2	25.2	22.4	22.3	23.3	13.0	11.9	08.0	07.0	13.23
	22.5	15.7	08.5	20.7	17.3	24.8	15.4	08.0	16.5	07.0	08.0	9.56	10.45
	41.0	10.2	13.6	16.6	37.8	17.4		06.0	14.2	10.3	13.3	16.4	10.95
Four Wheelers	24.4	22.6	17.3	25.1	27.6	16.5		36.6	08.2	07.0	09.0	08.5	10.13
	52.3	35.4	18.5	20.6	27.0	12.0		27.2	08.3	13.2	12.0	06.0	13.00
	13.0	10.9	08.5	09.0	15.4	20.8		13.0	09.2	15.0	13.3	12.0	07.00
	12.7	21.4	10.9	17.5	18.2	13.3		11.5	11.2	07.0	07.4	06.1	8.92
	07.5	09.4	9.13	23.5	09.9	09.1		10.5	13.8	12.5	13.5	07.0	13.43
	32.3							19.0					
	15.2							13.2					
	37.0							19.0					
	12.1							22.0					
Buses	08.9												
	27.2	40.6						08.8	27.0				
	35.0	63.7						08.0					
	22.7	93.2						18.2					
	15.2							20.2					
Trucks	32.5							18.0					
	25.9							17.6					
	25.6							41.9					
	45.5							14.4					

Table 3.27: Showing the Details of Service Rating Time.

						Servic	e Rating	Time						
Type of Vehicles			Hassat	1 to Ban	galore			Bangalore to						
			-	_	-		-	Hassan						
	10.91	09.13	19.48	07.82	17.48	15.53	13.33	14.47	08.86	13.21	22.59	17.48		
	20.08	11.28	08.53	15.49	09.25	18.55	14.41	13.03	20.63	12.65	10.10	18.55		
	20.78	07.97	22.63	09.89	09.27	09.98	08.29	41.07	10.91	09.56	19.49	22.36		
Faur What law	10.98	06.23	13.03	12.02	20.63	43.28	16.03	09.13	37.82	23.32	20.78	15.43		
Four Wheelers	13.47	23.81	25.29	16.53	12.97	11.63	21.21	12.17	52.33	07.13	20.89	11.26		
	08.23	16.51	17.38	20.89	08.98	09.28	22.01	24.61	27.01	13.48	22.59	16.78		
	10.93	17.48	07.82	24.27	15.23	09.74	10.46	18.55	09.25	09.36	17.51	09.44		
	19.46	09.13	12.18	12.17	15.40	18.63	11.28	15.49	07.3	08.92	16.57			
	15.18	44.63												
	38.05	09.23						15.29						
	16.38	33.01						09.62						
	11.41	20.46						11.48						
	15.36	19.27						39.43						
Buses	18.43							13.23						
	33.46	29.58			<u> </u>		<u> </u>	38.42						
	19.43	21.33						41.66						
Trucks	25.63	27.46						27.30						
110000	22.89 40.73	39.11 17.28						18.89 25.63						
	40.75	1/.20						25.05						
								63.75						
								21.49						

CHAPTER 4 ANALYSIS OF FIELD DATA

4.1 General

The delay and waiting time of drivers in toll plaza depends on service time and arrival rate. The quick service time and number of toll booths can reduce the time wasted in the queue. The wasted time can be calculated and minimised by analysis of the observed data. By calculating the wasted time the performance of the servers can be analysed and also the delay in overall travel time can be found out.

4.2 Road Inventory

The road inventory reveals that the number of incoming lanes is two and it diverges into five lanes within the queuing area of the toll plaza. The pavement structure before the toll plaza is divided flexible pavement. The number of lanes is four of width 9m and median width is 2.5m. The type of a shoulder is earthen and shoulder width is 1.5m.

The queuing area is of rigid pavement. The width of the pavement is 30m, and the length of pavement on arrival and merging side is 250m. the number toll booth on each side is 5, length of toll booth is 2.4m and width is 3.6m.

4.3 Traffic Volume

In the present study the traffic count census is done as per IRC: 9-1972 "Traffic census on Non-Urban Roads". To take into account the randomness, the traffic volume study was carried out in short intervals (4 hour) at different hours of a day and at different days. The average traffic flow rate from Hassan to Bangalore is 375veh/hr and the average traffic flow rate from Bangalore to Hassan is 345 veh/hr.

4.4 Space Mean Speed

In the present study the space mean speed is found out as per IRC: 108-1996 "Guidelines for Traffic Prediction on Rural Highways". The drivers slow down their vehicle before the toll plaza to judge the lanes and to select the lanes so that they spend less time in the queue. If the traffic flow rate increases the space mean speed decreases. The space mean speed of vehicles approaching the near the toll booth from Hassan to Bangalore is 21 km/hr and from Bangalore to Hassan is 14 km/hr.

The space mean speed of vehicles at a certain distance away from the queuing area on both the directions was found out to be 34km/hr and 26 km/hr respectively.

4.5 Time Headway

Time headway is the difference between the time the front wheel of a vehicle arrives at a point on the highway and the time of the front wheel of the next vehicle arrives at the same point. Time headway can be used to predict the flow rate of vehicles on a section of roadway the field data observed shows that the inter-arrival time between vehicles was found out to be 10.5s (Bangalore to Hassan) and 10.2s (Hassan to Bangalore). Both the directions have equal headway indicating nearly equal flow of vehicles in both the directions. Hence this might be one of reason for having equal number of booths in both the directions (i.e, 5 on each side).

4.6 Arrival Rate

The data obtained from the arrival rate was analysed using Poisson distribution. The observed frequency (598) approximately equal to the theoretical frequency (597) as per the Poisson distribution. Hence vehicles arriving at a section of a highway simply follow Poisson distribution. Hence for any future studies for finding out arrival rate, Poisson distribution can be made use off.

4.7 Service Rate

In the present study, the average service time from Hassan to Bangalore is 18.02 seconds and from Bangalore to Hassan is 17.06 seconds. The observed data reveals the service rate to be almost equal and hence service rate can be considered as general (G).

4.8 Traffic Flow Theroy

One of the most important equation in traffic flow theory is given between traffic flow (q), traffic density (k) and speed (s).

q=k*s

In the present study, graphs are plotted to show the behaviour of vehicles on the road section. In the present study the queuing theory was modelled on M/G/1. M represents the arrival pattern follows Poisson distribution, G represents service pattern considered general and 1 represents a single booth selected for the analysis.

Table 4.1: Shows Speed Density data for M/G/1 Model for One Lane

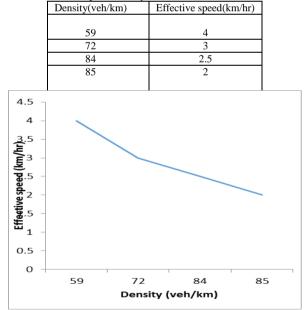


Figure 4.0: The speed density diagram for the M/G/1 model for one lane

Table 4.2: Shows Speed Density data for M/G/1 Model for Both Lanes

Density(veh/km)	Effective speed(veh/km)
18	11.1
26	8.14



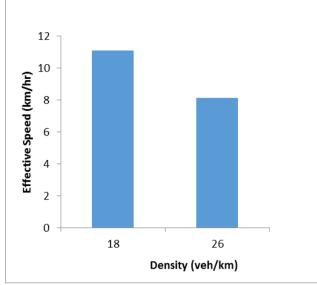


Figure 4.1: The speed density diagram for the M/G/1 model for both lanes

The above figure clearly indicates that as the traffic density increases the effective speed decreases. Hence speed and traffic density varies inversely. The travel time will also increase as density increases. The above bar graph shows that density on (Bangalore- Hassan) is more and the effective speed on that road section is less compared to (Hassanbangalore) where in the density is less and the effective speed is more.

Table 4.3: Shows the Effective Speed and Traffic Flow data

Traffic flow(veh/hr)	Effective speed(km/hr)
345	8.14
345	11.1

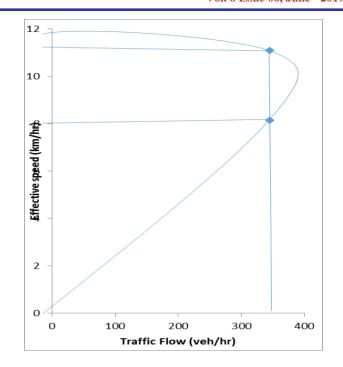


Figure 4.1: The speed flow diagram for the M/G/1

The speed flow figure is envelope of all possible combination of the effective speed and traffic flow. The effective speed decreases with increases traffic flow.

CONCLUSION

A detailed study was carried out to analyse the performance of a tool booth. The following conclusion was drawn from the observed data.

- It was found that flow rate remained constant on both directions 375 and 345 (veh/hr)
- The inter arrival time between two vehicle was found out be 10 seconds on both the directions.
- The waiting time in the queuing area was found out to be 10 seconds as general.
- The flow theory diagram reveals that as the density increases the effective speed decreases on that road section.

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